

Mechanics of Solids MMME2053

Finite Element Analysis Lecture 5

Learning Objectives

- 5. Understand how shape functions allow us to determine displacement (and stresses and strains within an element)
- 6. Understand how 2D approximations can be used to simplify the modelling of 3D problems
- 7. Understand how (simple 2D) continuum elements allow analysis of structures

- Although problems are 3D, we can make some assumptions and use a 2D approach for some problems
- Useful assumptions include the **plane stress, plane strain** and **axisymmetric** assumptions

- **Plane Stress** Approximation
- Consider a thin plate which is only loaded in the in-plane directions
- The normal stress σ _z must be zero on the front and back faces
- Because the plate is thin, then we can assume that $\sigma_z \approx 0$ throughout the thickness

• **Plane Stress** Approximation

• The only non-zero components of stress are σ_{x} , σ_{y} and τ_{xy} and we can determine all of the strain components, i.e. *εx*, *εy*, *ε^z* and *γxy*, from these stress components using Hooke's law (for elastic behaviour)

$$
\varepsilon_x = \frac{1}{E} (\sigma_x - \nu \sigma_y) \qquad \varepsilon_z = \frac{-\nu}{E} (\sigma_x + \sigma_y)
$$

$$
\varepsilon_x = \frac{1}{E} (\sigma_y - \nu \sigma_x) \qquad \gamma_{xy} = \frac{\tau_{xy}}{G}
$$

• **Plane Strain** Approximation

- Consider a very thick plate or long member of regular cross-section, again only loaded in the in-plane directions
- A plane ABCD, remote from the ends experiences negligible strain in the z-direction *εz* ≈ 0
- We can determine the z-direction stresses from the x and y-direction normal stresses

$$
\sigma_{z} = \nu(\sigma_{x} + \sigma_{y})
$$

• **Axisymmetric** Approximation

- Used to represent cases with geometry and loading that is rotationally symmetric (*r*, *z*, *θ* coordinates)
- Because of symmetry about the z axis, the stresses are independent of the *θ* coordinate

• **Axisymmetric** Approximation

• All derivatives with respect to *θ* vanish and the displacement component in the *θ* direction, the shear strains $γ_{rθ}$ and $γ_{θz}$ and the shear stresses $\tau_{r\theta}$ and $\tau_{\theta z}$ are all zero

• Plane Stress:

plate with hole

plate with fillet

• Plane Stress:

• Axisymmetric:

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Constant Strain Triangle (1)

- The simplest 2D element
- A 3-noded, linear triangular element, 2 DOF per node
- Linear variation of displacements within the element:

 $u_x(x, y) = C_1 + C_2 x + C_3 y$ $u_y(x, y) = C_4 + C_5x + C_6y$

Constant Strain Triangle (2)

• The constants $C_1 - C_6$ are related to the nodal coordinates and the values of the displacements by:

$$
\begin{pmatrix} u_{x1} \\ u_{y1} \\ u_{x2} \\ u_{y2} \\ u_{x3} \end{pmatrix} = \begin{bmatrix} 1 & x_1 & y_1 & 0 & 0 & 0 \\ 0 & 0 & 0 & 1 & x_1 & y_1 \\ 1 & x_2 & y_2 & 0 & 0 & 0 \\ 0 & 0 & 0 & 1 & x_2 & y_2 \\ 1 & x_3 & y_3 & 0 & 0 & 0 \\ 0 & 0 & 0 & 1 & x_3 & y_3 \end{bmatrix} \begin{bmatrix} C_1 \\ C_2 \\ C_3 \\ C_4 \\ C_5 \\ C_6 \end{bmatrix}
$$

 ${u} = [A]{C}$

Constant Strain Triangle (3)

• The strain field is calculated by:

• All constant values - no variation within the element – hence the name

Constant Strain Triangle (4)

• Which in matrix form is:

$$
\begin{Bmatrix} \varepsilon_x \\ \varepsilon_y \\ \gamma_{xy} \end{Bmatrix} = \begin{bmatrix} 0 & 1 & 0 & 0 & 0 & 0 \\ 0 & 0 & 0 & 0 & 0 & 1 \\ 0 & 0 & 1 & 0 & 1 & 0 \end{bmatrix} \begin{bmatrix} C_1 \\ C_2 \\ C_3 \\ C_4 \\ C_5 \\ C_6 \end{bmatrix}
$$

 $\{\varepsilon\} = [X]\{C\}$

Constant Strain Triangle (5)

• From before:

$$
\{C\} = [A]^{-1}\{u\}
$$

• So:

$$
\{\varepsilon\} = [X][A]^{-1}\{u\} = [B]\{u\}
$$

Constant Strain Triangle (6)

• If you work through the maths:

$$
[B] = \frac{1}{2A} \begin{bmatrix} y_2 - y_3 & 0 & y_3 - y_1 & 0 & y_1 - y_2 & 0 \ 0 & x_3 - x_2 & 0 & x_1 - x_3 & 0 & x_2 - x_1 \ x_3 - x_2 & y_2 - y_3 & x_1 - x_3 & y_3 - y_1 & x_2 - x_3 & y_1 - y_2 \end{bmatrix} \begin{bmatrix} C_1 \\ C_2 \\ C_3 \\ C_4 \\ C_5 \\ C_6 \end{bmatrix}
$$

• These are all known quantities as they are all a function of the coordinates of the nodes

Constant Strain Triangle (7)

- To relate stresses to strains, we require a matrix of elastic constants (Hooke's law)
- For plane stress cases (as mentioned before) this is (in matrix form):

$$
\begin{pmatrix} \varepsilon_x \\ \varepsilon_y \\ \gamma_{xy} \end{pmatrix} = \begin{bmatrix} \frac{1}{E} & -\frac{\nu}{E} & 0 \\ -\frac{\nu}{E} & \frac{1}{E} & 0 \\ 0 & 0 & \frac{2(1+\nu)}{E} \end{bmatrix} \begin{pmatrix} \sigma_x \\ \sigma_y \\ \tau_{xy} \end{pmatrix}
$$

Constant Strain Triangle (8)

• If we rearrange this:

 $\{\sigma\}=[D]\{\varepsilon\}$

• Where D (for plane stress) is:

$$
[D] = \frac{E}{1 - \nu^2} \begin{bmatrix} 1 & \nu & 0 \\ \nu & 1 & 0 \\ 0 & 0 & \frac{1 - \nu}{2} \end{bmatrix}
$$

Constant Strain Triangle (9)

• Substituting for the strains

$$
\{\sigma\}=[D][B]\{u\}
$$

• Which allows us to determine the element stresses from the element displacements

- A steel cantilever beam of length 0.5m width 0.01m and depth 0.025m, is subjected to a point load at the end of 5kN, determine the maximum stress in the beam and the maximum deflection of the tip of the cantilever.
- *E* = 200 GPa

CST Example

N_{el} = 10 (1 through thickness)

$$
\sigma_{x_max} = 1082 \text{ MPa}
$$

 $u_{y_min} = -0.0378$ m

CST Example

N_{el} = 20 (2 through thickness)

$$
\sigma_{x_max} = 1176 \text{ MPa}
$$

 $u_{y_min} = -0.0408$ m

CST Example

N_{el} = 1000 (5 through thickness)

$$
\sigma_{x_max} = 2195 \text{ MPa}
$$

$$
u_{y_{\text{min}}}
$$
 = -0.0703 m

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